

Influence of complex damping on transverse and longitudinal vibrations of portal frame

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Abstract. This paper presents a formulation and solution for the problem of transverse and longitudinal vibration of the portal frame with damping. The dissipation of vibration energy in the model is caused by simultaneous constructional, internal and external damping. The eigenvalues of the system for a selected and variable parameters were calculated. The effect of different type of damping on amplitude decay factor was presented.

Keywords: viscous damping, damped vibration, portal frame, amplitude decay factor.

1. Introduction

The dynamic properties of frames with additional discrete elements and crack, or loaded by the force were investigated in works [1-8]. In these works, authors formulated and solved the problem of stability and / or free and forced vibration of different type of frame without damping in considered system. In papers [1] and [2] the analysis of vibrations and stability of loaded Γ and T type frames were presented. In [3] was presented the results of research the influence of elastically restrained of supports on eigenfrequencies of frame. The influence of a crack depth and location on the vibrations of frame were presented in works [4-6]. The eigenfrequencies of a Γ type frame with additional discrete elements and crack was presented in paper [4]. The analysis of free vibrations of the 3D model of cracked open frame using FEM was proposed in work [5]. In paper [6], authors were studied the influence of bifurcation load magnitude and crack size on vibration frequency and amplitude of frame. Authors of work [7] analysed the vibrations of a multi-bar and elastically ends frames where the all frame elements were inclined. An analytical and numerical dynamic analysis of planar frame structures were presented in work [8]. In the real systems the vibration damping phenomenon always exist. Taking into account the different source of energy dissipation in mathematical model of real systems help determinates the dynamic properties of investigated systems. The formulation and solution for the problem of damped vibration of Γ frame is demonstrated in the study [9]. In that case only constructional damping were considered. Work [10] describes the problem of transverse vibrations of cracked frame with constructional and internal damping.

This study was concerned on a simultaneous influence of the impact of complex damping (constructional, external and internal) on longitudinal and transverse vibration of the portal frame. The dissipation of vibration energy in the model, derives from the constructional damping in the supports, from internal damping of viscoelastic material of the frame and from viscous medium surrounding the system. The formulation of free vibration issue was based on Hamilton's principle.

2. Physical model of the system

In adopted model, illustrated in Fig. 1, the energy dissipation is the consequence of resistance to motion in frame supports C_R , viscoelastic material of frame E_i^* and medium surrounding the system C_E (still fresh water).

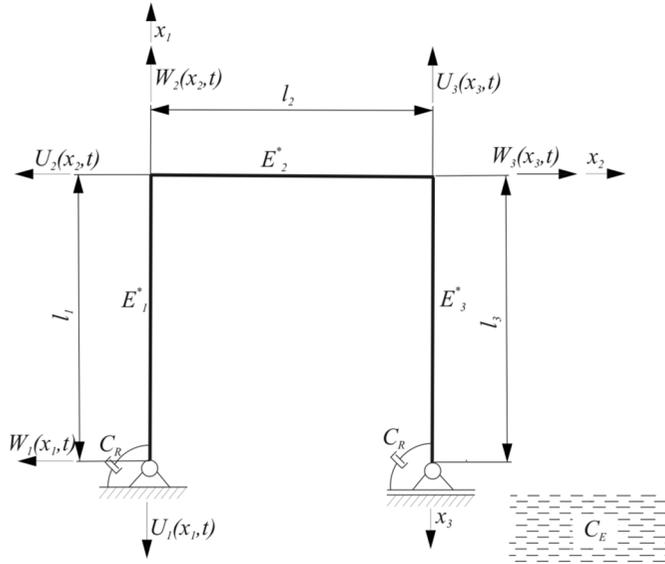


Fig. 1. Scheme of the considered system

3. Mathematical model and boundary problem

Basis on the Hamilton's principle the boundary problem for Bernoulli-Euler beams was formulated:

$$\delta \int_{t_1}^{t_2} (T - V) dt + \int_{t_1}^{t_2} \delta W_N dt = 0, \quad (1)$$

$$T = \frac{1}{2} \sum_{i=1}^3 \frac{\rho_i F_i}{2} \int_0^{l_i} \left(\frac{\partial W_i(x_i, t)}{\partial t} \right)^2 dx_i, \quad (2)$$

$$V = \frac{1}{2} \sum_{i=1}^3 \left[\int_0^{l_i} \frac{J_i E_i}{2} \left(\frac{\partial^2 W_i(x_i, t)}{\partial x_i^2} \right)^2 + \int_0^{l_i} \frac{F_i E_i}{2} \left[\left(\frac{\partial U_i(x_i, t)}{\partial x_i} \right) + \frac{1}{2} \left(\frac{\partial W_i(x_i, t)}{\partial x_i} \right)^2 \right] dx_i \right] \quad (3)$$

$$\begin{aligned} \delta W_N = & \sum_{i=1}^3 \left[\int_0^{l_i} E_i^* J_i \left[\frac{\partial^2}{\partial x_i^2} \left(\frac{\partial W_i(x_i, t)}{\partial t} \right) \right] \delta \frac{\partial^2 W_i(x_i, t)}{\partial x_i^2} dx_i \right. \\ & \left. + \int_0^{l_i} E_i^* F_i \left[\frac{\partial^2}{\partial x_i^2} \left(\frac{\partial W_i(x_i, t)}{\partial t} \right) \right] \delta \frac{\partial^2 W_i(x_i, t)}{\partial x_i^2} dx_i \right] - \int_0^{l_i} C_E \frac{\partial W_i(x_i, t)}{\partial t} \delta W_i(x_i, t) dx_i \\ & - \left[\left. C_R \left(\frac{\partial}{\partial t} \right) \frac{\partial W_1(x_1, t)}{\partial x_1} \right|_{x_1=0} \left. \delta \frac{\partial W_1(x_1, t)}{\partial x_1} \right|_{x_1=0} dx_1 \right. \\ & \left. + C_R \left(\frac{\partial}{\partial t} \right) \frac{\partial W_3(x_3, t)}{\partial x_3} \right|_{x_3=l_3} \delta \frac{\partial W_3(x_3, t)}{\partial x_3} \right|_{x_3=l_3} dx_3 \right], \quad (4) \end{aligned}$$

where: T – kinetic energy, V – potential energy, δW_N – virtual work of non-conservative forces, $W_i(x, t)$ – transverse displacements, $U_i(x, t)$ – longitudinal displacements, E_i – Young's modulus,

F_i – cross-sectional areas, J_i – moment of inertia in cross-sections, l_i – length of the beams, L_C – total length of frame, ρ_i – material density, x_i – spatial coordinate, t – time, $i = 1, 2, 3$.

The continuity and geometrical boundary conditions are expressed as:

$$\begin{aligned} u_1(0) = 0, \quad w_1(0) = 0, \quad w_1^I(l_1) = w_2^I(0), \quad w_1(l_1) = u_2(0), \quad u_1(l_1) = -w_2(0), \\ w_2^I(l_2) = w_3^I(0), \quad w_2(l_2) = u_3(0), \quad u_2(l_2) = -w_3(0), \quad u_3(l_3) = 0, \quad w_3(l_3) = 0, \end{aligned} \quad (5)$$

After placing to (1), Eq. (2)-(4) and taking into account the conditions Eq. (5), obtained the two equations of motion (in transversal and longitudinal direction):

$$J_i \left(E_i + E_i^* \frac{\partial}{\partial t} \right) \frac{\partial^4 W_i(x_i, t)}{\partial x_i^4} + C_E \frac{\partial W_i(x_i, t)}{\partial t} + \rho_i F_i \frac{\partial^2 W_i(x_i, t)}{\partial t^2} = 0, \quad (6)$$

$$-F_i \left(E_i + E_i^* \frac{\partial}{\partial t} \right) \frac{\partial^2 U_i(x_i, t)}{\partial x_i^2} + \rho_i F_i \frac{\partial^2 U_i(x_i, t)}{\partial t^2} = 0, \quad (7)$$

and natural boundary conditions:

$$\begin{aligned} (E_1 + E_1^* j \omega^*) J_1 w_1^{II}(0) = C_{R1} j \omega^* w_1^I(0), \quad (E_3 + E_3^* j \omega^*) J_3 w_3^{II}(l_3) = -C_{R3} j \omega^* w_3^I(l_3), \\ (E_1 + E_1^* j \omega^*) J_1 w_1^{II}(l_1) - (E_2 + E_2^* j \omega^*) J_2 w_2^{II}(0) = 0, \\ (E_2 + E_2^* j \omega^*) J_2 w_2^{II}(l_2) - (E_3 + E_3^* j \omega^*) J_3 w_3^{II}(0) = 0, \\ (E_1 + E_1^* j \omega^*) F_1 u_1^I(l_1) - (E_2 + E_2^* j \omega^*) J_2 w_2^{II}(0) = 0, \\ (E_2 + E_2^* j \omega^*) J_2 w_2^{II}(l_2) + (E_3 + E_3^* j \omega^*) F_3 u_3^I(0) = 0, \\ (E_2 + E_2^* j \omega^*) F_2 u_2^I(l_2) - (E_3 + E_3^* j \omega^*) J_3 w_3^{II}(0) = 0, \\ (E_1 + E_1^* j \omega^*) J_1 w_1^{III}(l_1) + (E_2 + E_2^* j \omega^*) F_2 u_2^I(0) = 0. \end{aligned} \quad (8)$$

The Eqs. (6) and (7) are solved as follows:

$$W_i(x_i, t) = w_i(x_i) e^{j\omega^* t}, \quad (9)$$

$$U_i(x_i, t) = u_i(x_i) e^{j\omega^* t}. \quad (10)$$

Substitution of Eqs. (9) and (10) into Eqs. (6) and (7) leads to, respectively:

$$w_i^{IV}(x_i) - \gamma_i w_i(x_i) = 0, \quad (11)$$

$$u_i^{II}(x_i) + \alpha_i^2 u_i(x_i) = 0, \quad (12)$$

where:

$$\gamma_i = \frac{\rho_i F_i}{J_i (E_i + E_i^* j \omega^*)} \left(\omega^{*2} - \frac{C_E}{\rho_i F_i} j \omega^* \right), \quad \alpha_i = \frac{\rho_i \omega^{*2}}{(E_i + E_i^* j \omega^*)}. \quad (13)$$

The solution of Eqs. (11) and (12) are expressed in the form of functions:

$$w_i(x) = B_{1i} e^{\lambda_i x} + B_{2i} e^{-\lambda_i x} + B_{3i} e^{j\lambda_i x} + B_{4i} e^{-j\lambda_i x}, \quad (14)$$

$$u_i(x) = B_{5i} e^{j\delta_i x} + B_{6i} e^{-j\delta_i x}, \quad (15)$$

where:

$$\omega^* = \text{Re}(\omega^*) \pm \text{Im}(\omega^*) j, \quad j = \sqrt{-1}, \quad \delta_i = \sqrt{\alpha_i}, \quad \lambda_i = \sqrt{\gamma_i}. \quad (16)$$

Introducing Eqs. (14) and (15) to boundary conditions Eqs. (5) and (8) leads to homogeneous

set of equations (B_{ki} – unknown constants), which can be written as matrix:

$$[A](\omega^*) B = 0, \quad (17)$$

where:

$$A(\omega^*) = [a_{pq}], \quad (p, q = 1, 2, \dots, 18), \quad B = [B_{ki}]^T, \quad k = 1, 2, \dots, 6. \quad (18)$$

If the determinant of the coefficient matrix equals to zero, considered system has a non-trivial solution (complex eigenvalues ω^*), in form:

$$\det A(\omega^*) = 0. \quad (19)$$

4. Parameters of the system

Computations were carried out for the parameters: beams length $l_1 = 1.4$ [m]; $l_2 = 2$ [m]; $l_3 = 1.4$ [m]; beams diameter $D_i = 0.05$ [m]; Young modulus $E_i = 2.1 \cdot 10^{11}$ [Pa]; material density $\rho_i = 7.7 \cdot 10^3$ [kg/m³].

The damping parameters have the following non-dimensional form.

For internal damping:

$$\eta = \frac{E_i^*}{h E_i'} \quad (20)$$

$$h^2 = \frac{\rho_i A_i}{E_i J_i} L_C^4. \quad (21)$$

For constructional damping:

$$\mu = \frac{c_R}{d}. \quad (22)$$

For external damping:

$$\nu = \frac{C_e L_C^3}{d}, \quad (23)$$

$$d^2 = L_C^2 (\rho_i A_i E_i J_i), \quad L_C = l_1 + l_2 + l_3. \quad (24)$$

For proportion between the length of the beams:

$$L = \frac{(l_1 + l_2)}{l_3}. \quad (25)$$

The main impact on damped vibration frequency and on amplitude decay factor have the values of μ from 0 to 2 (see [9, 10]). The value of η of homogeneous steel was selected according to data from works [11-13]. External damping comes from viscous medium surrounding the considered frame. Value of external damping of still fresh water is $C_E = 1.0041 \cdot 10^{-3}$ [Ns/m²] or $\nu = 0.49 \cdot 10^{-6}$ [11, 12, 14].

5. Results

The effect of the computations are presented in 2D and 3D graphs on Figs. 2-16. In Fig. 2. the first two natural frequencies of the frame (ω_1) and (ω_2) in relation to length L are illustrated. Presented relationships are the reference results. The comparison of undamped system (Fig. 2)

with damped system (Fig. 3) confirmed that, $Re(\omega^*)$ described the damped vibration frequency and $Im(\omega^*)$ described the amplitude decay factor. Changes between the value of real part of the first two eigenvalues and the value of first two natural frequencies derived from movement resistance in the supports of frame.

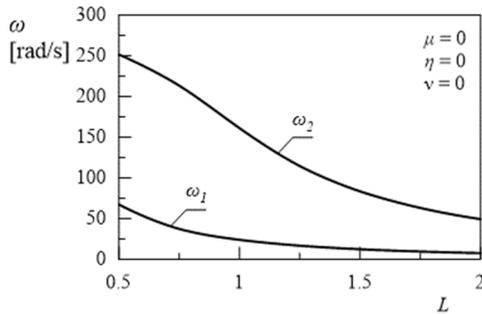


Fig. 2. Dependency between first two natural frequencies of frame and length L ($\eta = 0, \mu = 0, \nu = 0$)

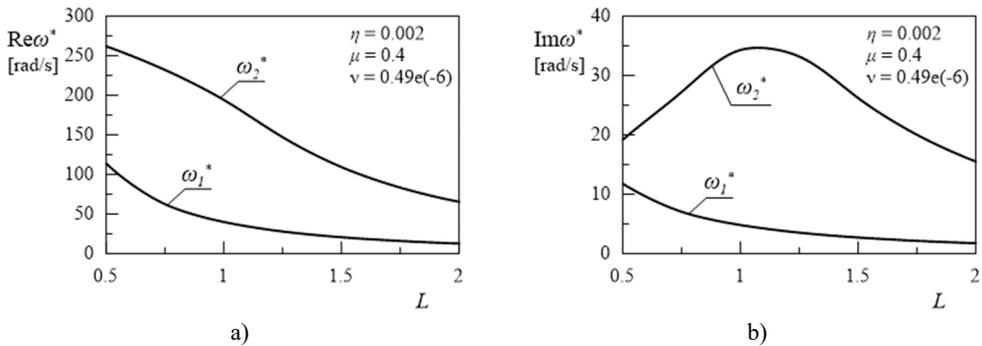


Fig. 3. Dependency between first and second eigenvalue and length L ($\eta = 0.002, \nu = 0.49e(-6), \mu = 0.4$)

The aim of this paper was to analyze of an influence of different kinds of damping on vibration of portal frame. The effect of changes the geometrical parameters on eigenvalues of damped system were also examined. To compare the influence of external, internal and constructional damping on frame eigenvalues calculations were conducted and relationships between damping coefficients and length of beams modelled frame have been determined.

Figs. 4-7 illustrated the influence of simultaneous changes in the length and constructional damping coefficient on eigenvalues of considered system.

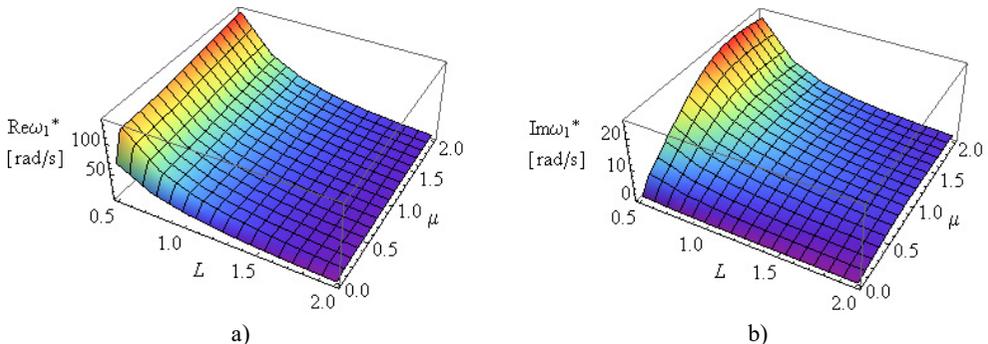


Fig. 4. Influence of L and coefficient μ on $Re(\omega_1^*)$ and $Im(\omega_1^*)$ ($\eta = 0.002, \nu = 0.49e(-6)$)

In Figs. 5 and 7 can be observed some substantial changes in the amplitude decay factor of

first and the second eigenvalue with changes the value of coefficient μ . The imaginary part increasing to maximum values, for selected values of μ and L , followed by $Im(\omega^*) \rightarrow 0$ when $\mu \rightarrow \infty$ (Figs. 4-7). Changes in $Re(\omega^*)$ illustrated in Figs. 4 and 6 are induced by the intervention in boundary conditions of system. When the value of coefficient μ is the highest the resistance and friction in the support (caused by a high value of viscous resistance) “locked” the rotary motion. On the three-dimensional graphs presented below the transition from joint mountings to rigid mountings can be observed.

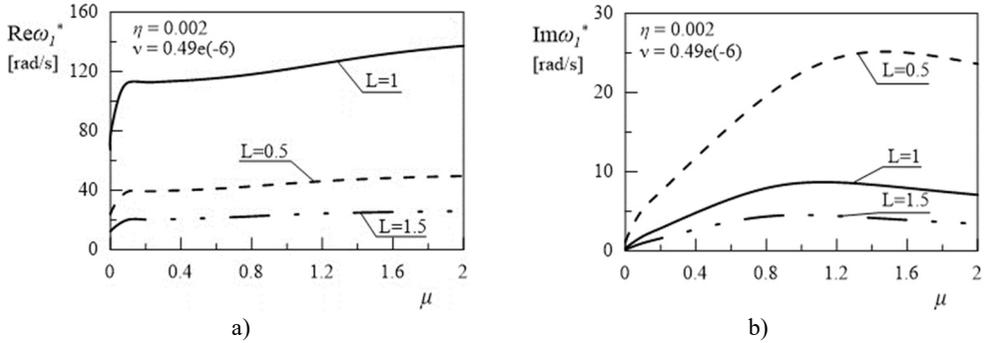


Fig. 5. Dependency between first eigenvalue and coefficient μ ($\eta = 0.002$, $\nu = 0.49e(-6)$)

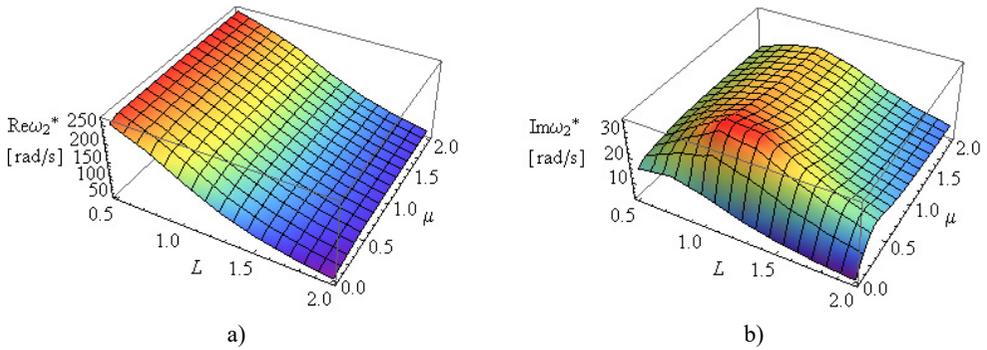


Fig. 6. Influence of L and coefficient μ on $Re(\omega_2^*)$ and $Im(\omega_2^*)$ ($\eta = 0.002$, $\nu = 0.49e(-6)$)

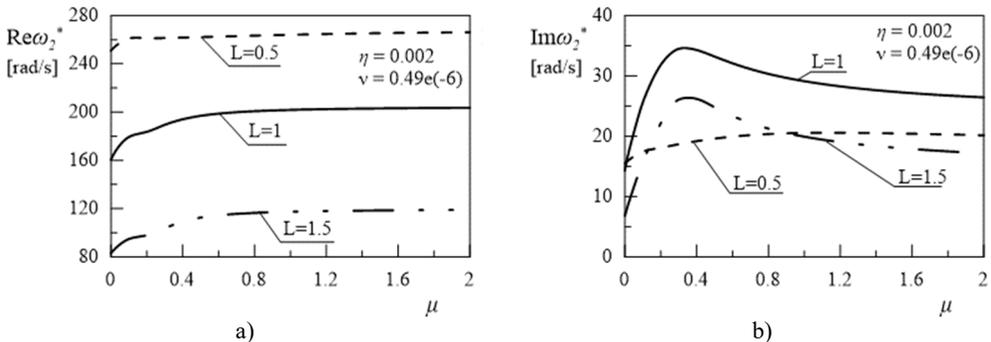


Fig. 7. Dependency between second eigenvalue and coefficient μ ($\eta = 0.002$, $\nu = 0.49e(-6)$)

Figs. 8-11 presents the results of the investigations of relationships between the first eigenvalue of the frame and internal damping coefficient η . Introduction of the internal damping to considered system has insignificant influence on the first and the second damped vibration frequency (Figs. 9 and 11). Significant changes can be observed in $Im(\omega_1^*)$ and $Im(\omega_2^*)$ with changes the value of coefficient η (Figs. 8-11). The increase in the values of amplitude decay factor throughout the

investigated range is caused by higher values of coefficient η .

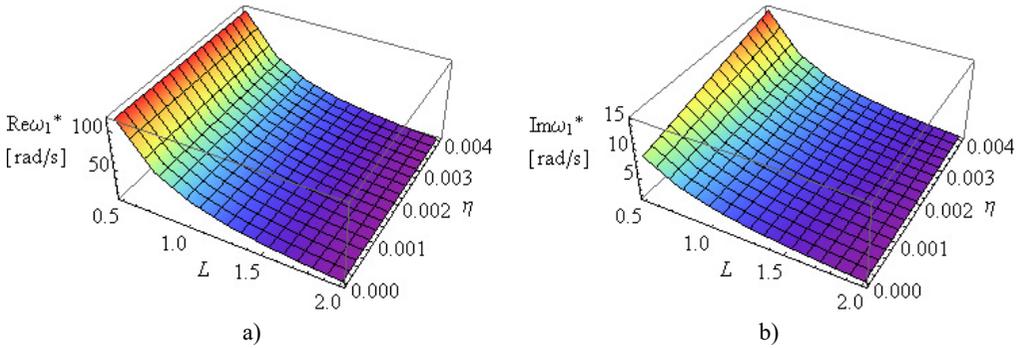


Fig. 8. Influence of L and coefficient η on $Re(\omega_1^*)$ and $Im(\omega_1^*)$ ($\mu = 0.4, \nu = 0.49e(-6)$)

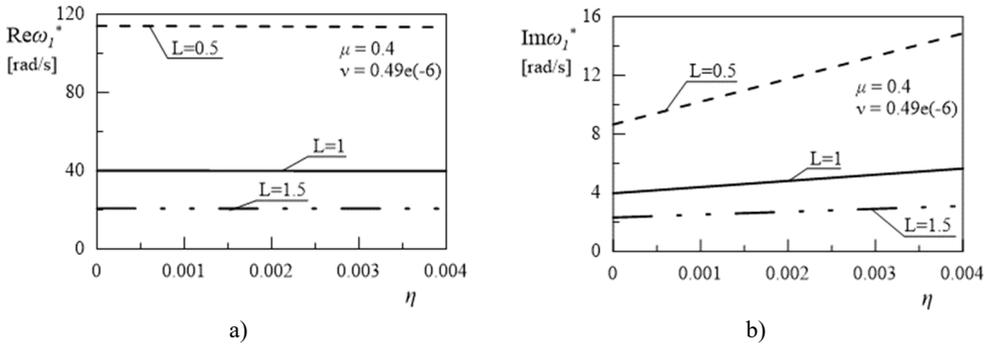


Fig. 9. Dependency between first eigenvalue and coefficient η ($\mu = 0.4, \nu = 0.49e(-6)$)

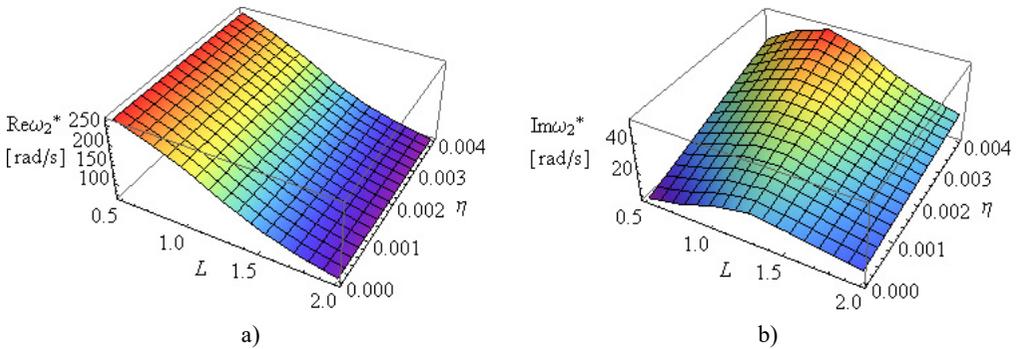


Fig. 10. Influence of L and coefficient η on $Re(\omega_2^*)$ and $Im(\omega_2^*)$ ($\mu = 0.4, \nu = 0.49e(-6)$)

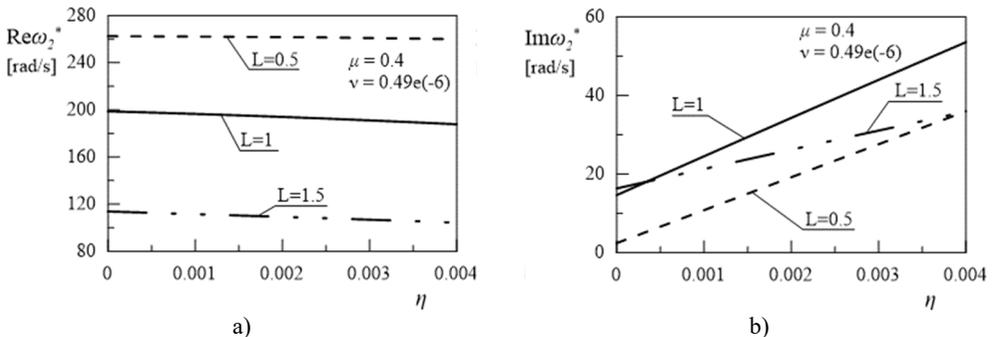


Fig. 11. Dependency between second eigenvalue and coefficient η ($\mu = 0.4, \nu = 0.49e(-6)$)

The obtained results helps to determine the proportion between the length of the beams L for which the amplitude decay factor reaches the maximum value. Fig. 12 and 13 illustrated the dependency between the coefficient ν and $Re(\omega^*)$ and $Im(\omega^*)$ of the first and the second eigenvalue for frame. Only high value of external damping has significant effects on values of imaginary part of solution. Influence of coefficient ν on damped frequency $Re(\omega^*)$ in Figs. 12 and 13 is similar to influence of coefficient η on $Re(\omega^*)$ in Figs. 9 and 11.

The comparison of the effect of complex damping on frame eigenvalues, leads to the conclusion, that the main influence on vibration amplitude has the values of constructional damping coefficient (Figs. 4-7). The progression of function $Im(\omega^*)$, with changes the value of coefficient μ , illustrated in Figs. 14 and 15 confirms that assumption.

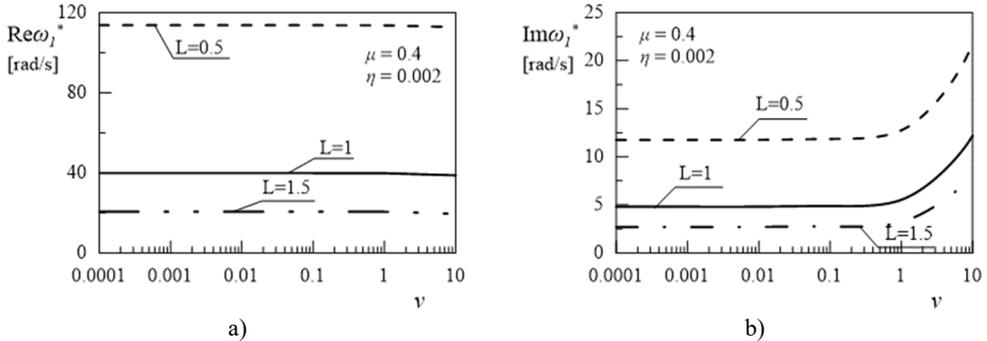


Fig. 12. Dependency between first eigenvalue and coefficient ν ($\mu = 0.4, \eta = 0.002$)

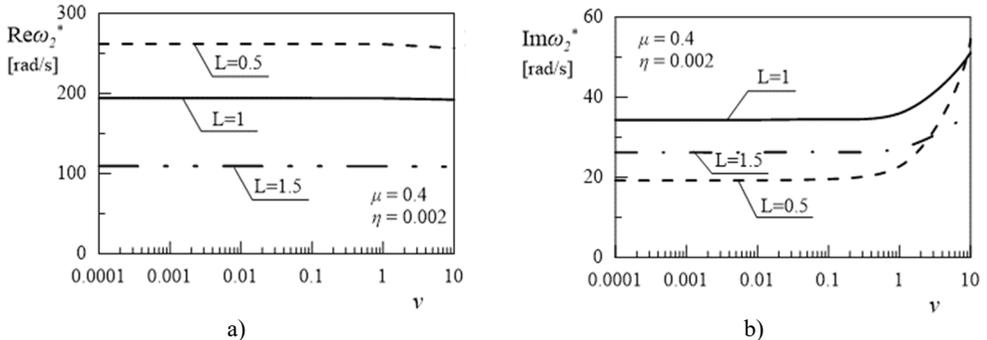


Fig. 13. Dependency between second eigenvalue and coefficient ν ($\mu = 0.4, \eta = 0.002$)

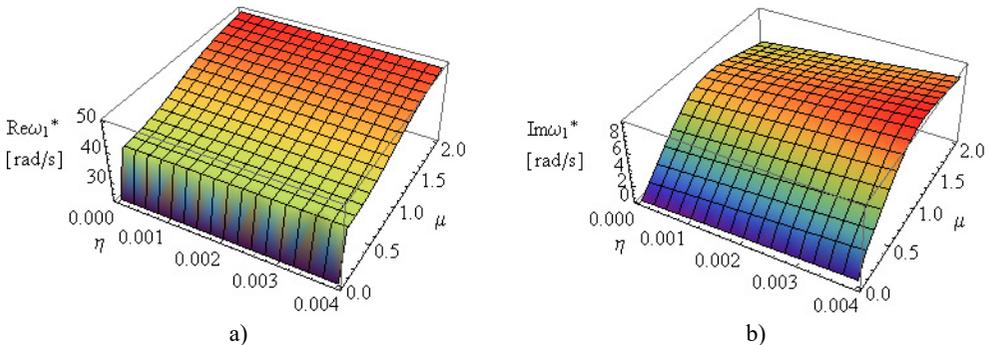


Fig. 14. Influence of coefficients μ and η on $Re(\omega^*)$ and $Im(\omega_1^*)$ ($L = 2, \nu = 0.49e(-6)$)

The higher value of $Im(\omega^*)$ then the amplitudes of vibration are more damped, as can be seen in the all above figures. In Fig. 16 the maximum values of amplitude decay factor for the two first

modes of vibration in the examined system depending on the length L are presented.

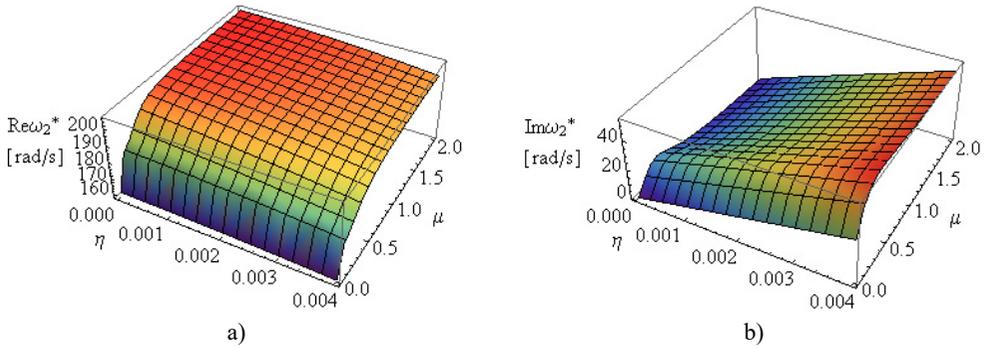


Fig. 15. Influence of coefficients μ and η on $Re(\omega_2^*)$ and $Im(\omega_2^*)$ ($L = 2, \nu = 0.49e(-6)$)

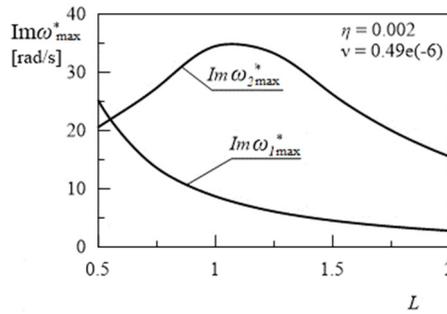


Fig. 16. Dependency between the $Im(\omega_{max}^*)$ and the length L (for $\eta = 0.002, \nu = 0.49e(-6)$)

6. Conclusions

In this paper, the influence of internal, external and constructional damping on vibration of portal frame have been examined. The results presented in work shown, that taken into consideration the dissipation of vibration energy (as a result of different types of damping) to the mathematical model of vibrations of portal frame, causes substantial influence on the damped frequencies and vibration amplitude of the considered system. The density of medium surrounding the system has significant influence on values of amplitude decay factor and high value of external damping coefficient leads to the increasing of $Im(\omega^*)$. The still fresh water resistance has a minor impact of frame vibration. The increase in the value of internal damping coefficient causes considerable increase in the amplitude decay factor while the insignificant changes in the damped frequency can be observed. The comparison of the effect of complex damping on frame eigenvalues, leads to the conclusion, that the main influence on vibration amplitude of investigated system was exerted the constructional damping. Changes between the value of real part of the first two eigenvalues and the value of first two natural frequencies derived from movement resistance – the rotary motion in supports is locked and the transition from joint to rigid mountings can be observed.

The obtained results helps determine the geometric parameters of frame and values of the damping coefficients for which the minimum vibration amplitudes in the system are maintained i.e. helps to determine the proportion between the length of the beams L for which the amplitude decay factor reaches the maximum value. The method described in this work could be useful as an additional design tool for engineers to modelled multi-bar systems with minimal vibrations amplitude. Based on the obtained results it is also possible to reduce the amplitude of vibration using rotational viscous dampers.

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