# 163. Some possibilities of modeling of moving load on concrete pavements

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Abstract. The submitted paper is devoted to the numerical modeling of the dynamic effects of moving vehicles on concrete pavements. The finite element method, namely the ADINA program, is used to model the problem. To solve the task, it is necessary to create a vehicle and a road computational models. The vehicle model is spacious and corresponds to a heavy truck of the Tatra type. The so-called single-layer road model on an elastic Winkler foundation is used. Concrete pavements are modeled using shell or solid elements. The output of the solution is the time course of the dynamic deflections at the individual points of the slab and internal forces and the stress states in the concrete slab at the individual time points of the solution. The calculation results for each computational model are compared to each other.

**Keywords:** moving load, concrete pavements, computing models, ADINA, dynamic analysis.

#### 1. Introduction

Roads are typical transport structures subjected to the dynamic effect of moving vehicles. The most widespread types of roads are asphalt and concrete pavements. The moving load effect on pavements can be analyzed by numerical or experiment way. The determination of the dynamic response of road pavements to moving vehicle loads is a subject of current interest and importance, which are deals in many departments around the world [1-3]. To solve the task by the numerical way, it is necessary to create a vehicle and a road computational models. The models can be created in the sense of classical dynamic or in the sense of finite element method. The finite element method, namely the ADINA program [4], is used to model the problem in this paper. The vehicle model is spacious and corresponds to a heavy truck of the Tatra type. The road model can be created at different qualitative levels. The so-called single-layer road model on an elastic Winkler foundation is used. Concrete pavements can be modeled using shell or solid elements. Shell elements are more preferred in the case if the internal forces are needed to design the structure. The solid elements are more advantageous if the output is to be a more detailed analysis of the stress states of the structure. The base eight nodal elements may be used, or more nodal elements can be used depending on what output is expected from the solution. The output of the solution is the time course of the dynamic deflections at the individual points of the slab and internal forces and the stress states in the concrete slab at the individual time points of the solution. The calculation results for each computational model are compared to each other.

#### 2. Vehicle computational model

One of the most important steps for proper numerical simulation is to choose the appropriate model of the vehicle. The model should preferably represent the real vehicle. For the purposes of this simulation, the spatial model of the whole vehicle was chosen, Fig. 1. This numerical model of the vehicle has finite degrees of freedom and it is a combination of masses, springs, damping and 3D solid element. The numerical parameters used in this numerical model are as follows:

Geometry: a = 3.135 m; b = 1.075 m; c = 0.660 m; d = 0.993 m; e = 0.973 m. Diagonal stiffness matrix:  $[k_i]_D = [k_1; k_2; k_3; k_4; k_5; k_6; k_7; k_8; k_9; k_{10}]_D = [143716.5;$ 

143716.5; 761256; 761256; 1275300; 1275300; 2511360; 2511360; 2511360; 2511360]<sub>D</sub> [N/m] . Diagonal mass matrix:  $[m]_D = [m_1; I_{y1}; I_{x1}; m_2; m_3; m_4; I_{y4}; m_5; I_{y5}]_D = [22950.0; 62298.0;$ 

22950.0; 455; 455; 1070; 466; 1070; 466]<sub>D</sub> [kg, kg·m<sup>2</sup>].

Diagonal damping matrix:  $[b_i]_D = [b_1; b_2; b_3; b_4; b_5; b_6; b_7; b_8; b_9; b_{10}]_D = [9614; 9614; 130098.5; 130098.5; 1373; 1373; 2747; 2747; 2747; 2747]_D [kg/s].$ 

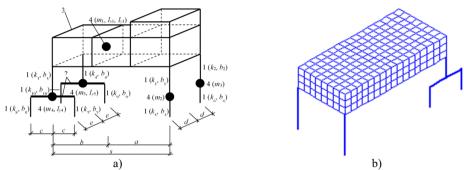


Fig. 1. Vehicle computational model

### 3. Road computational model

The subject of the modeling, in this case, is a concrete pavement consisting of nine slabs with planar dimensions of  $6.8\times4.9$  m and a thickness of 0.24 m located in three rows next to each other. The total dimension of this area is  $20.4\times14.7$  m. The solved area is covered by a finite element network measuring  $0.425\times0.490$  m. The network represents 30 rows of elements parallel to the *X*-axis and 48 rows of elements parallel to the *Y*-axis. The pavement composition in cross-section is shown in Fig. 2.

Concrete Slab; 240 mm; 
$$E = 37.500$$
 MPa;  $\nu = 0.20$ ;  $\rho = 2.500$  kg/m<sup>3</sup>

Coating Aggregate II; 40 mm;  $E = 4.500$  MPa;  $\nu = 0.21$ ;  $\rho = 2.400$  kg/m<sup>3</sup>

Soil Cement I; 200 mm;  $E = 2.000$  MPa;  $\nu = 0.23$ ;  $\rho = 1.800$  kg/m<sup>3</sup>

Protective Coat; 250 mm;  $E = 120$  MPa;  $\nu = 0.35$ ;  $\rho = 1.700$  kg/m<sup>3</sup>

Sub-Soil;  $\infty$  mm;  $E = 60$  MPa;  $\nu = 0.35$ ;  $\rho = 1.800$  kg/m<sup>3</sup>

Fig. 2. Pavement composition in cross-section

FEM computational models of concrete pavements can be created as a single layer or multilayer models. Single layer models are sufficient in case we want to analyze stress and strain states only in the reinforced concrete slab itself and we are not interested in stress and strain states in the individual subgrade layers. In this paper the single layer models of concrete pavement on elastic Winkler foundation are adopted, Fig. 3.

The modulus of compressibility k [MN/m<sup>3</sup>] was calculated using the LAYMED program as follows [5]:

$$k = 1.274 \frac{p}{w_p} = 1.274 \frac{0.10}{0.000471228} = 270.3574 \text{ MN/m}^3(x).$$

The layered half-space was loaded through an elastic steel circular plate, radius r=40 cm, with the pressure p=0.10 MPa. The value of the surface deflection of the considered layered half-space calculated by LAYMED is  $w_p=0.471228$  mm. Because the computed value of the deflection corresponds to the load transmitted by the elastic plate, the deflection must be recalculated to the rigid plate by a correction coefficient of 1.274. The elastic foundation is replaced by equivalent springs located in nodes of the FEM network. The area corresponding to one node is  $A=0.425\cdot0.49=0.20828$  m². The stiffness of the spring located in the node is  $K=k\cdot A=270.3574\cdot0.20828=56.301928\cong56.3$  MN/m = 56.300.000 N/m. The mass intensity of the plate  $\mu$  [kg/m²] is calculated as  $\mu=\rho\cdot h=2500\cdot0.24=600.0$  kg/m². The damping is introduced into the calculation by the angular damping frequency in the value  $\omega_b=0.1$  rad/s.

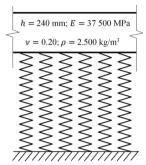
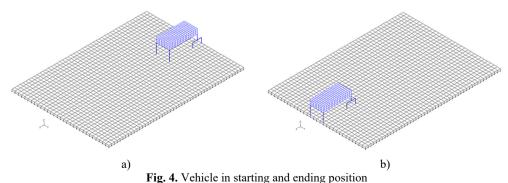


Fig. 3. Sigle layer computational model

The concrete slab can be modeled by a shell or solid elements. The Shell element is more advantageous in case if the outputs are to be the internal forces needed to dimensioning the slab (bending moments, shear forces, torsional moments). The solid element is more advantageous in case if the outputs are to be stress and strain states in the slab.

## 4. Results of the computation

The properties of the concrete slab are as follows: slab thickness h = 240 mm, modulus of elasticity of concrete  $E = 37\,500$  MPa, Poisson's ratio v = 0.20, bulk density  $\rho = 2500$  kg/m<sup>3</sup>. Other pavement layers are introduced into the calculation as an elastic Winkler foundation with compressibility modulus k = 270.3574 MN/m<sup>3</sup>. The stiffness of the spring located in the node is K = 56.300.000 N/m. The damping is introduced into the calculation as Rayleigh's damping with  $\alpha = 0.1$  and  $\beta = 0.002$ .



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The vehicle moves along the slab central axis with constant speed V = 65 km/h. The vehicle

in starting and ending positions is shown in Fig. 4. Vehicle passage time along the slab is 0.86 s. The computation of the slab and vehicle response at riding of the vehicle along the slab was calculated in the environment of FEM System ADINA [4].

### 4.1. Model with shell elements

The used shell element has six DOFs in each node (three translational displacements in the X, Y and Z directions, and three rotational deformations with respect to the X, Y and Z axes). The output of the calculation is vertical displacements and internal forces. The time course of the vertical displacements in the middle of the slab at the speed of vehicle motion V = 65 km/h is shown in Fig. 5. Maximal displacement occurs at time t = 1.541 s in the value  $w_{max} = 0.063775 \text{ mm}$ . Calculation time is 15 minutes.

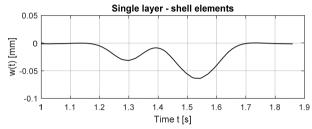


Fig. 5. Time course of the vertical displacements in the middle of the slab, shell elements

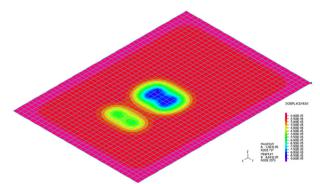


Fig. 6. Vertical displacement in time t = 1.541 s, shell elements

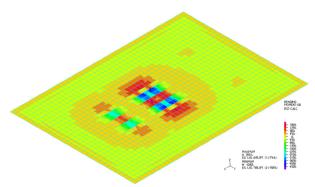


Fig. 7. Bending moments  $m_{yy}$  parallel to the X-axis at time t = 1,541 s, shell elements

The isostructural surface of vertical displacements at the time t = 1.541 s when the maximal deflection in the middle of the slab occurs is shown in Fig. 6. The isostructural surface of bending

moment  $m_{yy}$  parallel to the X-axis at time t = 1.541 s when a maximal deflection in the middle of the slab occurs is shown in Fig. 7.

### 4.2. Model with solid elements

The used solid element has eight nodes with three DOFs in each node, total 24 DOFs. The output of the calculation is vertical displacements, stresses and contact force. The time course of the vertical displacements in the middle of the slab at the speed of vehicle motion V = 65 km/h is shown in Fig. 8. Maximal displacement occurs at time t = 1.544 s in the value  $w_{max} = 0.059536 \text{ mm}$ . Calculation time is 7 minutes. The isostructural surface of vertical displacements at the time t = 1.544 s when the maximal deflection in the middle of the slab occurs is shown in Fig. 9. The isostructural surface of normal stresses  $\sigma_{zz}$  at time t = 1.544 s when a maximal deflection in the middle of the slab occurs is shown in Fig. 10. The corresponding contact forces are shown in Fig. 11.

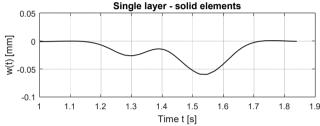


Fig. 8. Time course of the vertical displacements in the middle of the slab, solid elements

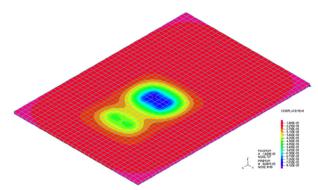


Fig. 9. Vertical displacement in time t = 1.544 s, solid elements

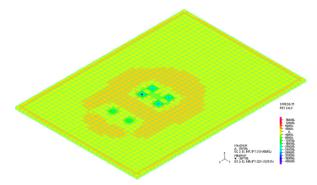
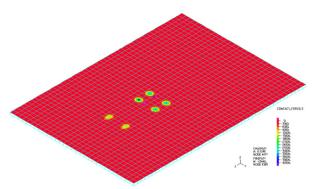


Fig. 10. Normal stresses  $\sigma_{zz}$  at the time t = 1.544 s, solid elements



**Fig. 11.** Contact force at the time t = 1.544 s, solid elements

## 4.3. Comparison of shell and solid element models

The results for FEM computational models using the shell and solid elements were mutually compared. The subject of comparison is the time course of vertical deflections in the middle of the slab. Comparison in graphical form is shown in Fig. 12. Comparison in numerical form is in Table 1. The solid model is a bit stiffer than the shell model.

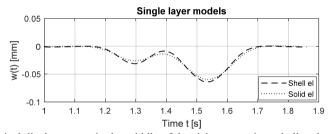


Fig. 12. Vertical displacements in the middle of the slab, comparison shell and solid elements

Table 1. Comparison of maximal vertical deflection of the slab for shell and solid element models

	Shell	Solid	Solid – Shell	% of Shell
t [s]	1.541	1.544	+ 0.003	+ 0.194
$w_{max}$ [mm]	0.063	0.059	-0.004	- 6.646

#### 5. Conclusions

Vehicle-road interaction is the actual engineering problem, solving in many workplaces. The current state of computing technique allows solving the problem in a numerical way in real time. The problem can be modeled using the finite element method in the environment of a commercial software, e.g. ADINA. It is advisable to use the spatial model of vehicle. The road model can be created as a single layer or multilayer model. Single layer models are sufficient in case we want to analyze stress and strain states only in the reinforced concrete slab itself and we are not interested in stress and strain states in the individual subgrade layers. The complexity of the model must be adapted to the quality of the results. Shell elements are preferred in the case if the internal forces are needed to design the structure. The solid elements are advantageous if the output is to be a more detailed analysis of the stress states of the structure. In both cases, the contact forces between the wheel and the road can also be obtained. It is best to combine numerical and experimental procedures together and to verify numerical results. The use of such procedures may have different applications in practice [6-8].

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